Standard Practice for Design and Installation of Groundwater Monitoring Wells[1]

This standard is issued under the fixed designation D5092/D5092M; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ε) indicates an editorial change since the last revision or reapproval.

1. Scope*

1.1 This practice describes a methodology for designing and installing conventional (screened and filter-packed) groundwater monitoring wells suitable for formations ranging from unconsolidated aquifers (that is, sands and gravels) to granular materials having grain-size distributions with up to 50 % passing a #200 sieve and as much as 20 % clay-sized material (that is, silty fine sands with some clay). Formations finer than this (that is, silts, clays, silty clays, clayey silts) can be monitored but the well may not yield sufficient water required for sampling, and fine filter pack and screen requirements are difficult and costly to install. Use of coarser filter/screens in fine formations will result in wells with unstable filter packs and associated elevated sample turbidity that may adversely affect sample accuracy and data quality objectives. This practice is not applicable in fractured or karst rock conditions, but may be applicable for other porous rock formations.

1.2 The recommended monitoring well design and installation procedures presented in this practice are based on the assumption that the objectives of the program are to obtain representative groundwater samples and other representative groundwater data from a targeted zone of interest in the subsurface defined by site characterization.

1.3 This practice when used on coarse grained sand and gravel aquifers, in combination with proper well development (D5521), proper groundwater sampling procedures (D4448), and proper well maintenance and rehabilitation (D5978), will permit acquisition of groundwater samples free of artifactual turbidity, eliminate siltation of wells between sampling events, and permit acquisition of accurate groundwater levels and hydraulic conductivity test data from the zone screened by the well. For wells installed in fine-grained formation materials, it is generally necessary to use much finer pre-packed well screens (6.3.3.2) and/or employ sampling methods that minimize screen intake flow velocity, and disturbance of the well column including suspension of settled solids in the well. Using low-flow purging and sampling techniques (D6771) or passive sampling devices (D7929) are two means to minimize the potential sample bias associated with turbidity.

1.4 This practice applies primarily to well design and installation methods used in drilled boreholes. Other standards, including Guide D6724 and Practice D6725, cover installation of monitoring wells using direct-push methods.

1.5 *Units*—The values stated in either inch-pound units or SI units [presented in brackets] are to be regarded separately as standard. The values stated in each system may not be exact equivalents; therefore, each system shall be used independently of the other. Combining values from the two systems may result in non-conformance with the standard. Equivalent values given in parentheses are shown for mix designs and sieves sizes.

1.5.1 Sieve Designations (Specification E11) are identified using the "alternate" system, for example, #40, #200 sieve etc. with nominal opening size in inches and particle sizes in mm. See Specification E11 for standard metric sieve sizes.

1.5.2 Well screen slots are expressed in inches and the metric equivalent is given in the terminology section and when necessary in the standard (see 3.3.6).

1.6 All observed and calculated values shall conform to the guidelines for significant digits and rounding established in Practice D6026, unless superseded by this standard.

1.7 *This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.*

1.8 *This practice offers a set of instructions for performing one or more specific operations. This document cannot replace education or experience and should be used in conjunction with professional judgment. Nat all aspects of this practice may be applicable in all circumstances. This ASTM standard is not intended to represent or replace the standard of care by which the adequacy of a given professional service must be judged, nor should this document be applied without consideration of a project's many unique aspects. The word "Standard" in the title of this document means only that the document has been approved through the ASTM consensus process.*

[1] This practice is under the jurisdiction of ASTM Committee D18 on Soil and Rock and is the direct responsibility of Subcommittee D18.21 on Groundwater and Vadose Zone Investigations.

Current edition approved Nov. 15, 2016. Published December 2016. Originally approved in 1990. Last previous edition approved in 2010 as D5092–04(2010)$^{\varepsilon 1}$. DOI: 10.1520/D5092_D5092M-16.

***A Summary of Changes section appears at the end of this standard**

2. Referenced Documents

2.1 *ASTM Standards:*[2]

C150 Specification for Portland Cement
C294 Descriptive Nomenclature for Constituents of Concrete Aggregates
D422 Test Method for Particle-Size Analysis of Soils (Withdrawn 2016)[3]
D653 Terminology Relating to Soil, Rock, and Contained Fluids
D1129 Terminology Relating to Water
D1452 Practice for Soil Exploration and Sampling by Auger Borings
D1586 Test Method for Penetration Test (SPT) and Split-Barrel Sampling of Soils
D1587 Practice for Thin-Walled Tube Sampling of Fine-Grained Soils for Geotechnical Purposes
D2113 Practice for Rock Core Drilling and Sampling of Rock for Site Exploration
D2487 Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System)
D2488 Practice for Description and Identification of Soils (Visual-Manual Procedure)
D3282 Practice for Classification of Soils and Soil-Aggregate Mixtures for Highway Construction Purposes
D3550 Practice for Thick Wall, Ring-Lined, Split Barrel, Drive Sampling of Soils (Withdrawn 2016)[3]
D3740 Practice for Minimum Requirements for Agencies Engaged in Testing and/or Inspection of Soil and Rock as Used in Engineering Design and Construction
D4448 Guide for Sampling Ground-Water Monitoring Wells
D5088 Practice for Decontamination of Field Equipment Used at Waste Sites
D5299 Guide for Decommissioning of Groundwater Wells, Vadose Zone Monitoring Devices, Boreholes, and Other Devices for Environmental Activities
D5434 Guide for Field Logging of Subsurface Explorations of Soil and Rock
D5518 Guide for Acquisition of File Aerial Photography and Imagery for Establishing Historic Site-Use and Surficial Conditions
D5521 Guide for Development of Groundwater Monitoring Wells in Granular Aquifers
D5608 Practices for Decontamination of Sampling and Non Sample Contacting Equipment Used at Low Level Radioactive Waste Sites
D5753 Guide for Planning and Conducting Borehole Geophysical Logging
D5777 Guide for Using the Seismic Refraction Method for Subsurface Investigation
D5778 Test Method for Electronic Friction Cone and Piezocone Penetration Testing of Soils
D5781 Guide for Use of Dual-Wall Reverse-Circulation Drilling for Geoenvironmental Exploration and the Installation of Subsurface Water-Quality Monitoring Devices
D5782 Guide for Use of Direct Air-Rotary Drilling for Geoenvironmental Exploration and the Installation of Subsurface Water-Quality Monitoring Devices
D5783 Guide for Use of Direct Rotary Drilling with Water-Based Drilling Fluid for Geoenvironmental Exploration and the Installation of Subsurface Water-Quality Monitoring Devices
D5784 Guide for Use of Hollow-Stem Augers for Geoenvironmental Exploration and the Installation of Subsurface Water-Quality Monitoring Devices
D5787 Practice for Monitoring Well Protection
D5872 Guide for Use of Casing Advancement Drilling Methods for Geoenvironmental Exploration and Installation of Subsurface Water-Quality Monitoring Devices
D5875 Guide for Use of Cable-Tool Drilling and Sampling Methods for Geoenvironmental Exploration and Installation of Subsurface Water-Quality Monitoring Devices
D5876 Guide for Use of Direct Rotary Wireline Casing Advancement Drilling Methods for Geoenvironmental Exploration and Installation of Subsurface Water-Quality Monitoring Devices
D5978 Guide for Maintenance and Rehabilitation of Groundwater Monitoring Wells
D6001 Guide for Direct-Push Groundwater Sampling for Environmental Site Characterization
D6026 Practice for Using Significant Digits in Geotechnical Data
D6067 Practice for Using the Electronic Piezocone Penetrometer Tests for Environmental Site Characterization
D6167 Guide for Conducting Borehole Geophysical Logging: Mechanical Caliper
D6169 Guide for Selection of Soil and Rock Sampling Devices Used With Drill Rigs for Environmental Investigations
D6274 Guide for Conducting Borehole Geophysical Logging - Gamma
D6282 Guide for Direct Push Soil Sampling for Environmental Site Characterizations
D6285 Guide for Locating Abandoned Wells
D6286 Guide for Selection of Drilling Methods for Environmental Site Characterization
D6429 Guide for Selecting Surface Geophysical Methods
D6430 Guide for Using the Gravity Method for Subsurface Investigation
D6431 Guide for Using the Direct Current Resistivity Method for Subsurface Investigation
D6432 Guide for Using the Surface Ground Penetrating Radar Method for Subsurface Investigation
D6519 Practice for Sampling of Soil Using the Hydraulically Operated Stationary Piston Sampler
D6639 Guide for Using the Frequency Domain Electromagnetic Method for Subsurface Investigations
D6640 Practice for Collection and Handling of Soils Obtained in Core Barrel Samplers for Environmental Investigations

[2] For referenced ASTM standards, visit the ASTM website, www.astm.org, or contact ASTM Customer Service at service@astm.org. For *Annual Book of ASTM Standards* volume information, refer to the standard's Document Summary page on the ASTM website.

[3] The last approved version of this historical standard is referenced on www.astm.org.

D6724 Guide for Installation of Direct Push Groundwater Monitoring Wells
D6725 Practice for Direct Push Installation of Prepacked Screen Monitoring Wells in Unconsolidated Aquifers
D6771 Practice for Low-Flow Purging and Sampling for Wells and Devices Used for Ground-Water Quality Investigations (Withdrawn 2011)[3]
D6914 Practice for Sonic Drilling for Site Characterization and the Installation of Subsurface Monitoring Devices
D7242 Practice for Field Pneumatic Slug (Instantaneous Change in Head) Tests to Determine Hydraulic Properties of Aquifers with Direct Push Groundwater Samplers
D7929 Guide for Selection of Passive Techniques for Sampling Groundwater Monitoring Wells
D8037 Practice for Direct Push Hydraulic Logging Profiling Variations of Permeability Soils
E11 Specification for Woven Wire Test Sieve Cloth and Test Sieves
F480 Specification for Thermoplastic Well Casing Pipe and Couplings Made in Standard Dimension Ratios (SDR), SCH 40 and SCH 80

3. Terminology

3.1 *Definitions:*

3.1.1 For definitions of common technical terms in this standard, refer Terminology D653.

3.1.2 *artifactual turbidity, n—in wells, filters*, particulate matter that is not naturally mobile in the groundwater system and that is produced in some way by the groundwater sampling process.

3.1.2.1 *Discussion*—May consist of particles introduced to the subsurface during drilling or well construction, sheared from the target monitoring zone during pumping or bailing the well, or produced by exposure of groundwater to atmospheric conditions.

3.1.3 *ballast, n—in drilling*, materials used to provide stability to a buoyant object (such as casing within a water-filled borehole).

3.1.4 *borehole, n—in drilling*, an open or uncased subsurface hole, generally circular in plain view, created by drilling.

3.1.5 *borehole log, n—in drilling*, the record of geologic units penetrated, drilling progress, depth, water level, sample recovery, volumes, and types of materials used, and other significant facts regarding the drilling and/or installation of an exploratory borehole or well.

3.1.6 *bridge, n—in drilling*, an obstruction within the annulus that may prevent circulation or proper placement of annular fill materials.

3.1.7 *casing, n—in drilling*, pipe, finished in sections with either threaded connections or beveled edges to be field welded, which is installed temporarily or permanently either to counteract caving, to advance the borehole, or to isolate the zone being monitored, or any combination of these.

3.1.8 *casing, protective, n—in drilling*, a section of larger diameter pipe that is placed over the upper end of a smaller diameter monitoring well riser or casing to provide structural protection to the well, to prevent damage to the well, and to restrict unauthorized access into the well.

3.1.9 *casing, surface, n—in drilling*, pipe used to stabilize a borehole near the surface during the drilling of a borehole that may be left in place or removed once drilling is completed.

3.1.10 *caving; sloughing, v—in drilling*, the inflow of unconsolidated material into a borehole that occurs when the borehole walls lose their cohesiveness.

3.1.11 *cement, n—in drilling*, commonly known as Portland cement. A mixture that consists of calcareous, argillaceous, or other silica-, alumina-, and iron-oxide-bearing materials that is manufactured and formulated to produce various types which are defined in Specification C150. Portland cement is considered a hydraulic cement because it must be mixed with water to form a cement-water paste that has the ability to harden and develop strength even if cured under water.

3.1.12 *centralizer, n—in drilling*, a device that assists in the centering of a casing or riser within a borehole or another casing.

3.1.13 *confining unit, n—in hydrogeology*, a body of relatively low hydraulic conductivity formation material stratigraphically adjacent to one or more aquifers.

3.1.13.1 *Discussion*—Synonymous with or may include formations that are considered to be "aquiclude," "aquitard," and "aquifuge."

3.1.14 *flush joint or flush coupled, n—in drilling*, casing or riser with ends threaded such that a consistent inside and outside diameter is maintained across the threaded joints or couplings.

3.1.15 *gravel pack, n—in wells, filters*, common term used to refer to the primary filter pack of a well (see *primary filter pack*).

3.1.16 *hydrologic unit, n—in geology, hydrogeology*, geologic strata that can be distinguished on the basis of capacity to yield and transmit fluids. Aquifers and confining units are types of hydrologic units. Boundaries of a hydrologic unit may not necessarily correspond either laterally or vertically to lithostratigraphic formations.

3.1.17 *neat cement, n—in grouting*, a mixture of Portland cement (Specification C150) and water.

3.1.18 *piezometer, n—in wells, hydrogeology*, a small-diameter well with a very short screen that is used to measure changes in hydraulic head, usually in response to pumping a nearby well. Synonymous with observation well.

3.1.19 *piping, n*—the progressive removal of soil particles from a mass by percolating water, leading to the development of channels.

3.1.20 *primary filter pack, n—in wells*, a clean silica sand or sand and gravel mixture of selected grain size and gradation that is installed in the annular space between the borehole wall and the well screen, extending an appropriate distance above the screen, for the purpose of retaining and stabilizing the particles from the adjacent formation(s). The term is used in place of *gravel pack*.

3.1.21 *PTFE tape, n—in drilling*, joint sealing tape composed of polytetrafluoroethylene.

3.1.22 *riser, n—in wells*, the pipe or well casing extending from the well screen to just above or below the ground surface.

3.1.23 *secondary filter pack, n—in wells*, a clean, uniformly graded sand that is placed in the annulus between the primary filter pack and the overlying seal, or between the seal and overlying grout backfill, or both, to prevent intrusion of the seal or grout, or both, into the primary filter pack.

3.1.24 *sediment sump, n—in wells*, a blank extension of pipe or well casing, closed at the bottom, beneath the well screen used to collect fine-grained material from the filter pack and adjacent formation materials during the process of well development. Synonymous with rat trap or tail pipe.

3.1.25 *static water level, n—in hydrogeology*, the elevation of the top of a column of water in a monitoring well or piezometer that is not influenced by pumping or conditions related to well installation, or hydraulic testing.

3.1.26 *tamper, n—in piezometers and wells*, a heavy cylindrical metal section of tubing that is operated on a wire rope or cable. It either slips over the riser and fits inside the casing or borehole annulus, or fits between the riser and annulus. It is generally used to tamp annular sealants or filter pack materials into place and to prevent bridging or break bridges that form in the annular space.

3.1.27 *target monitoring zone, n—in geoenvironmental programs*, the groundwater flow path from a particular area or facility in which monitoring wells will be screened. The target monitoring zone should be an interval in subsurface materials in which there is a reasonable expectation that a monitoring well will intercept groundwater moving beneath an area or facility and any migrating contaminants that may be present.

3.1.28 *tremie pipe, n—in wells*, a small-diameter pipe or tube that is used to transport filter pack materials and annular seal materials from the ground surface into an annular space.

3.1.29 *uniformity coefficient, n—in soils*, the ratio of D_{60}/D_{10}, where D_{60} and D_{10} are particle diameters corresponding to 60 % and 10 % finer on the cumulative particle size curve, respectively.

3.1.30 *uniformly graded, n—in soils*, a quantitative definition of the particle size distribution of a soil that consists of a majority of particles being of approximately the same diameter. A granular material is considered uniformly graded when the uniformity coefficient is less than about five (Test Method D2487). Comparable to the geologic term *well sorted*.

3.1.31 *vented cap, n—in wells/piezometers*, a cap with a small hole that is installed on top of the riser.

3.1.32 *weep hole, n—in drilling*, a small-diameter hole (usually ¼ in.) drilled into the protective casing above the ground surface that serves to drain out water that may enter the annulus between the riser and the protective casing.

3.1.33 *well completion diagram, n—in wells*, a record that illustrates the details of a well installation.

3.1.34 *well screen, n—in wells*, a device used to retain the primary or natural filter pack; usually a cylindrical pipe with openings of a uniform width, orientation, and spacing.

3.2 *Terms Referenced in D1129, Committee D19 on Water:*

3.2.1 *turbidity, n*—expression of the optical properties of a sample that causes light rays to be scattered and absorbed rather than transmitted in straight lines through the sample. (Turbidity of water is caused by the presence of suspended and dissolved matter such as clay, silt, finely divided organic matter, plankton, other microscopic organisms, organic acids, and dyes.)

3.2.1.1 *Discussion*—The D19 definition is related to measurement of turbidity. For the purpose of this standard, turbidity is cloudiness or haziness in a fluid caused by the presence of small suspended solids that are otherwise imperceptible to the naked eye.

3.3 *Definitions of Terms Specific to This Standard:*

3.3.1 *annular space; annulus, n*—the space between two concentric strings of casing, or between the casing and the borehole wall. This includes the space(s) between multiple strings of casing in a borehole installed either concentrically or adjacent to one another

3.3.2 *grout (monitoring wells), n*—a low-permeability material placed in the annulus between the well casing or riser and the borehole wall (in a single-cased monitoring well), or between the riser and casing (in a multi-cased monitoring well), to prevent movement of groundwater or surface water within the annular space.

3.3.3 *multi-cased well, n*—a well constructed by using successively smaller diameter casings with depth.

3.3.4 *packer (monitoring wells)*—a transient or dedicated device placed in a well that isolates or seals a portion of the well, annulus, or borehole at a specific level.

3.3.5 *single-cased well, n*—a monitoring well constructed with a riser but without an exterior casing.

3.3.6 *slot, n—wells screen opening*, slot openings have been designated by numbers which correspond to the width of the openings in thousandths of an inch. A No. 10 slot screen, for example, is an opening of 0.010 inch [0.25 mm].

4. Significance and Use

4.1 This practice for the design and installation of groundwater monitoring wells will promote *(1)* efficient and effective site hydrogeological characterization; *(2)* durable and reliable well construction; and *(3)* acquisition of representative groundwater quality samples, groundwater levels, and hydraulic conductivity testing data from monitoring wells. The practices established herein are affected by governmental regulations and by site-specific geological, hydrogeological, climatological, topographical, and subsurface geochemical conditions. To meet these geoenvironmental challenges, this practice promotes the development of a conceptual hydrogeologic model prior to monitoring well design and installation.

NOTE 1—This practice presents a design for monitoring wells that will be effective in the majority of formations. This practice is in general accordance with other national and state guidance documents on well construction (ANSI/NGWA-01-14 (**1**)[4] and California EPA (**2**)) however;

[4] The boldface numbers in parentheses refer to a list of references at the end of this standard.

national, state, or local design regulations may control design and installation.

4.2 A properly designed and installed groundwater monitoring well provides essential information on one or more of the following subjects:

4.2.1 Formation geologic and hydraulic properties;

4.2.2 Potentiometric surface of a particular hydrologic unit(s);

4.2.3 Water quality with respect to various indicator parameters; and

4.2.4 Water chemistry with respect to a contaminant release.

NOTE 2—The quality of the result produced by this standard is dependent on the competence of the personnel performing it, and the suitability of the equipment and facilities used. Agencies that meet the criteria of Practice D3740 are generally considered capable of competent and objective testing/sampling/inspection/etc. Users of this standard are cautioned that compliance with Practice D3740 does not in itself assure reliable results. Reliable results depend on many factors; Practice D3740 provides a means of evaluating some of those factors.

Practice D3740 was developed for agencies engaged in the laboratory testing and/or inspection of soils and rock. As such, it is not totally applicable to agencies performing this practice. However, user of this practice should recognize that the framework of Practice D3740 is appropriate for evaluating the quality of an agency performing this practice. Currently there is no known qualifying national authority that inspects agencies that perform this practice. Use of certified water well drillers are recommended. There are national and state agencies that certify water well drillers.

5. Site Characterization

5.1 *General*—A thorough knowledge of site-specific geologic, hydrologic and geochemical conditions is necessary to properly apply the monitoring well design and installation procedures contained within this practice. Development of a conceptual site model, that identifies the target monitoring zone(s), and generates a three dimensional (3-D) picture of contaminant distribution and contaminant movement pathways, is recommended prior to monitoring well design and installation. Development of the conceptual site model is accomplished in two phases—an initial reconnaissance, after which a preliminary conceptual model is created, and a field exploration, after which a revised conceptual model is formulated. When the hydrogeology of a project area is relatively uncomplicated and well documented in the literature, the initial reconnaissance may provide sufficient information to identify flow paths and the target monitoring zone(s). However, where limited or no background data are available or where the geology is complex, a field exploration will be required to develop the necessary conceptual site model.

5.2 *Initial Reconnaissance of Project Area*—The goal of the initial reconnaissance of the project area is to identify and locate those zones or preferential flow pathways with the greatest potential to transmit fluids from the project area. Identifying these flow pathways is the first step in selecting the target groundwater monitoring zone(s).

5.2.1 *Literature Search*—Every effort should be made to collect and review all applicable field and laboratory data from previous explorations of the project area. Information such as, but not limited to, topographic maps, aerial imagery (see Guide D5518), site ownership and utilization records, geologic and hydrogeologic maps and reports, mineral resource surveys, water well logs, information from local well drillers, agricultural soil reports, geotechnical engineering reports, and other engineering maps and reports related to the project area should be reviewed to locate relevant site information.

5.2.2 *Field Reconnaissance*—Early in the exploration, the soil and rocks in open cut areas (for example, roadcuts, streamcuts) in the vicinity of the project should be studied, and various soil and rock profiles noted. Special consideration should be given to soil color and textural changes, landslides, seeps, and springs within or near the project area.

5.2.3 *Preliminary Conceptual Model*—The distribution of the predominant soil and rock units likely to be found during subsurface exploration may be hypothesized at this time in a preliminary conceptual site model using information obtained in the literature search and field reconnaissance. In areas where the geology is relatively uniform, well documented in the literature, and substantiated by the field reconnaissance, further refinement of the conceptual model may not be necessary unless anomalies are discovered in the well drilling stage.

5.3 *Field Exploration*—The goal of the field exploration is to refine the preliminary conceptual site model so that the target monitoring zone(s) is (are) identified prior to monitoring well installation.

5.3.1 *Exploratory Boreholes and Direct-Push Methods*—Characterization of the flow paths conceptualized in the initial reconnaissance involves defining the porosity (type and amount), hydraulic conductivity, stratigraphy, lithology, gradation and structure of each hydrologic unit encountered beneath the site. These characteristics are defined by conducting an exploratory program which may include, but not limited to: drilled boreholes (see Guide D6286 for selection of drilling methods) and direct-push methods (for example, cone penetrometers (see Test Method D5778 or Guide D6067) or direct-push machines using soil sampling, groundwater sampling and/or electrical conductivity measurement tools (see Guides D6001 and D6282; Practices D7242 and D8037). Exploratory boreholes and direct-push holes should be deep enough to develop the required engineering and hydrogeologic data for determining the preferential flow pathway(s), target monitoring zone(s), or both.

5.3.1.1 *Sampling*—Soil and rock properties should not be predicted wholly on field description or classification, but should be confirmed by laboratory and/or field tests made on samples or in boreholes or wells. Representative soil or rock samples of each material that is significant to the design of the monitoring well system should be obtained and evaluated by a geologist, hydrogeologist, soil scientist or engineer trained and experienced in soil and rock analysis. Soil sample collection should be conducted according to Practice D1452, Test Method D1586, Practice D3550, Guide D6282, Practice D6519 or Practice D1587, whichever is appropriate given the anticipated characteristics of the soil samples (see Guide D6169 for selection of soil sampling methods). Rock samples should be collected according to Practice D2113. Soil samples obtained for evaluation of hydraulic properties should be containerized and identified for shipment to a laboratory. Special measures to preserve either the continuity of the sample or the natural moisture are not usually required. However, soil and rock samples obtained for evaluation of chemical properties often require special field preparation and preservation to prevent significant alteration of the chemical constituents during transportation to a laboratory (see Practice D6640). Rock samples for evaluation of hydraulic properties are usually obtained using a split-inner-tube core barrel. Evaluation and logging of the core samples is usually done in the field before the core is removed from the core barrel.

5.3.1.2 *Boring Logs*—Care should be taken to prepare and retain a complete boring log and sampling record for each exploratory boring or direct-push hole (see Guide D5434).

NOTE 3—Site explorations conducted for the purpose of generating data for the installation of groundwater monitoring wells can vary greatly due to the availability of reliable site data or the lack thereof. The general procedure would be as follows: *(1)* gather factual data regarding the surficial and subsurface conditions, *(2)* analyze the data, *(3)* develop a conceptual model of the site conditions, *(4)* locate the monitoring wells based on the first three steps. Monitoring wells should only be installed with sufficient understanding of the geologic, and hydrologic and geochemical conditions present at the site. Monitoring wells often serve as part of an overall site exploration for a specific purpose, such as determining the extent of contamination present, or for predicting the effectiveness of aquifer remediation. In these cases, extensive additional geotechnical and hydrogeologic information may be required that would go beyond the Section 5, Site Characterization, description.

Boring logs should include the location, geotechnical data (that is, penetration rates or blow counts and sample intervals), and sample description information for each material identified in the borehole either by symbol or word description, or both. Description and identification of soils should be in accordance with Practice D2488; classification of soils should be in accordance with either Practice D2487 or Practice D3282. Identification of rock material should be based on Nomenclature C294 or by an appropriate geologic classification system. Observations of seepage, free water, and water levels should also be noted. The boring logs should be accompanied by a report that includes a description of the area investigated; a map illustrating the vertical and horizontal location (with reference to national vertical datum such as North American Vertical Datum of 1988 [NAVD 88] or to a standardized survey grid) of each exploratory borehole or test pit, or both; and color photographs of rock cores, soil samples, and exposed strata labeled with a date and identification.

5.3.2 *Geophysical Exploration*—Geophysical surveys may be used to supplement borehole and outcrop observation data and to aid in interpretation between borings. Appropriate surface and borehole geophysical methods for meeting site-specific project objectives can be selected by consulting Guides D6429 and D5753 respectively. Surface geophysical methods such as seismic (Guide D5777), electrical-resistivity (Guide D6431), ground-penetrating radar (Guide D6432), gravity (Guide D6430) and electromagnetic conductance surveys (Guide D6639) can be particularly valuable when distinct differences in the properties of contiguous subsurface materials are indicated. Borehole methods such as resistivity, gamma, gamma-gamma, neutron, and caliper logs (see Guide D6167) can be useful to confirm specific subsurface geologic conditions. Gamma logs (Guide D6274) are particularly useful in existing cased wells.

5.3.3 *Groundwater Flow Direction*—Groundwater flow direction is generally determined by measuring the vertical and horizontal hydraulic gradient within each conceptualized flow pathway. However, because water will flow along the pathways of least resistance in the highest hydraulic conductivity, most transmissive, formation materials at the site, actual flow direction may be oblique to the average hydraulic gradient (within buried stream channels or glacial valleys, for example). Flow direction is determined by first installing piezometers in the exploratory boreholes that penetrate the zone(s) of interest at the site. The depth and location of the piezometers will depend upon anticipated hydraulic connections between conceptualized flow pathways and their respective lateral direction of flow. Following careful evaluation, it may be possible to utilize existing private or public wells to obtain water-level data (Guide D6285). The construction integrity of such wells should be verified to ensure that the water levels obtained from the wells are representative only of the zone(s) of interest. Following water-level data acquisition, a potentiometric surface map should be prepared. Flow pathways are ordinarily determined to be at right angles, or nearly so, to the equipotential lines, though consideration of complex geology can result in more complex interpretations of flow

5.4 *Completing the Conceptual Model*—A series of geologic and hydrogeologic cross sections should be developed to refine the conceptual model. This is accomplished by first plotting logs of soil and rock observed in the exploratory soil boreholes or test pits, and interpreting between these logs using the geologic and engineering interrelationships between other soil and rock data observed in the initial reconnaissance or with geophysical techniques. Extrapolation of data into adjacent areas should be done only where geologically uniform subsurface conditions are known to exist. The next step is to integrate the geologic profile data with the potentiometric data for both vertical and horizontal hydraulic gradients. Plan view and cross-sectional flow nets should be constructed. Following the analysis of these data, conclusions can be made as to which flow pathway(s) is (are) the appropriate target monitoring zone(s).

6. Monitoring Well Construction Materials

6.1 *General*—The materials that are used in the construction of a monitoring well that come in contact with water samples should not alter the chemical quality of the sample for the constituents being examined. The riser, well screen, and annular seal installation equipment should be cleaned immediately prior to well installation (see either Practice D5088 or D5608) or certified clean from the manufacturer and delivered to the site in a protective wrapping.

6.2 *Water*—Water used in the drilling process, to prepare grout mixtures and to decontaminate the well screen, riser, and annular sealant injection equipment, should be obtained from a source of known chemistry that does not contain constituents that could compromise the integrity of the well installation. Water used in the process should be analyzed for the same analytes if required in the sampling plan.

6.3 *Primary Filter Pack:*

6.3.1 *General*—The purposes of the primary filter pack are to act as a filter that retains formation material while allowing groundwater to enter the well, and to stabilize the formation to keep it from collapsing on the well. The design of the primary filter pack is based on the grain-size distribution of the formation material (as determined by sieve analysis—see Test Method D422) to be retained (**3, 4, 5, and 6**). The grain size distribution of the primary filter pack must be fine enough to retain the formation, but coarse enough to allow for unrestricted movement of groundwater into and through the monitoring well. The design of the well screen (see 6.4.3 and Fig. 1) must be done in concert with the design of the filter pack. After development, a monitoring well with a correctly designed and installed filter pack and screen combination should produce samples free of artifactual turbidity.

6.3.2 *Materials*—The primary filter pack should consist of an inert granular material (generally ranging from gravel to very fine sand, depending on formation grain size distribution) of selected grain size and gradation that is installed in the annulus between the well screen and the borehole wall. Washed and screened silica sands and gravels, with less than 5 % non-siliceous materials, should be specified.

6.3.3 *Design*—The design theory of filter pack gradation is based on mechanical retention of formation materials.

6.3.3.1 *Coarse Grained Formations*—For formation materials that are relatively coarse-grained (that is, fine, medium and coarse sands and gravels), the grain size distribution of the primary filter pack is determined by calculating the D_{30} (30 % finer) size, the D_{60} (60 % finer) size, and the D_{10} (10 % finer) size of the filter pack. The first point on the filter pack grain-size distribution curve is the D_{30} size. The primary filter pack is usually selected to have a D_{30} grain size that is about 4 to 6 times greater than the D_{30} grain size of the formation material being retained (see Fig. 1). A multiplication factor of 4 is used if the formation material is relatively fine-grained and well sorted or uniform (small range in grain sizes); a multiplication factor of 6 is used if the formation is relatively coarse grained and poorly sorted or non-uniform (large range in grain sizes). Thus, 70 % of the filter pack will have a grain size that is 4 to 6 times larger than the D_{30} size of the formation

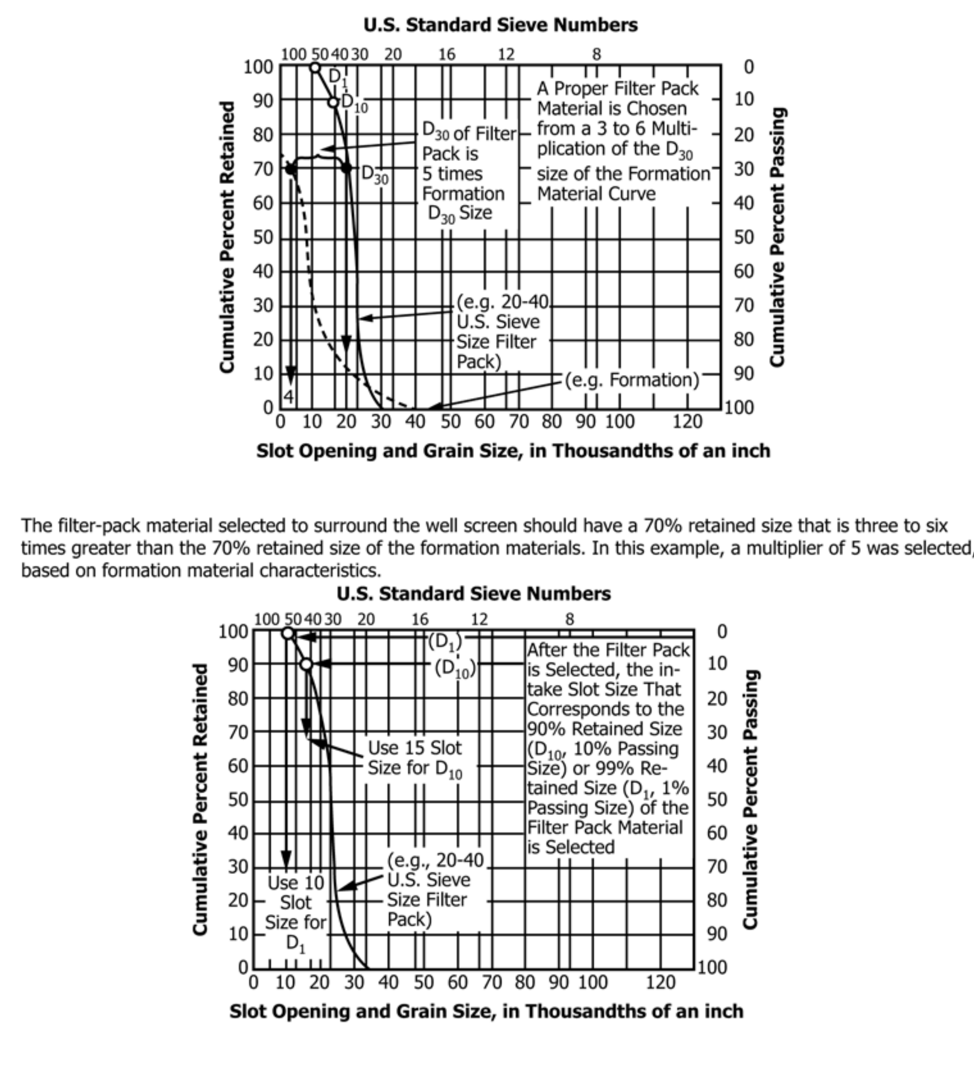


FIG. 1 Example Grading Curves for Design of Filter Pack and Slot Size (5)

materials. This ensures that the filter pack is coarser (with a higher hydraulic conductivity) than the formation material, and allows for unrestricted groundwater flow from the formation into the monitoring well.

(1) The next 2 points on the filter pack grain-size distribution curve are the D_{60} and D_{10} grain sizes. These are chosen so that the ratio between the two grain sizes (the uniformity coefficient) is less than 2.5. This ensures that the filter pack has a small range in grain sizes and is uniform (see technical Note 4). The D_{60} and D_{10} grain sizes of the filter pack are calculated by a trial and error method using grain sizes that are close to the D_{30} size of the filter pack. After the D_{30}, D_{60} and D_{10} sizes of the filter pack are determined, a smooth curve is drawn through these points. The final step in filter pack design is to specify the limits of the grain size envelope, which defines the permissible range in grain sizes for the filter pack. The permissible range on either side of the grain size curve is 8 %. The boundaries of the grain size envelope are drawn on either side of the filter pack grain-size distribution curve, and filter pack design is complete. For examples, see references (**3, 4, 5**). A filter medium having a grain-size distribution as close as possible to this curve is then obtained from a local sand supplier.

NOTE 4—Because the well screen slots have uniform openings, the filter pack should be composed of particles that are as uniform in size as is practical. Ideally, the uniformity coefficient (the quotient of the 60 % passing, D_{60} size divided by the 10 % passing D_{10} size [effective size]) of the filter pack should be 1.0 (that is, the D_{60} % and the D_{10} % sizes should be identical). However, a more practical and consistently achievable uniformity coefficient for all ranges of filter pack sizes is 2.5. This value of 2.5 should represent a maximum value, not an ideal.

6.3.3.2 *Fine-Grained Formations*—In formation materials that are predominantly fine-grained (finer than fine to very fine sands), soil piping can occur when a hydraulic gradient exists between the formation and the well (as would be the case during well development and sampling). To prevent soil piping in these materials, the following criteria are used for designing granular filter packs (**7**):

$$\frac{D_{15}\text{of filter}}{D_{85}\text{of formation}} \leq 4 \text{ to } 5 \text{ and} \frac{D_{15}\text{of filter}}{D_{15}\text{of formation}} \geq 4 \text{ to } 5$$

The left half of this equation is the fundamental criterion for the prevention of soil piping through a granular filter, while the right half of the equation is the hydraulic conductivity criterion. This latter criterion serves the same purpose as multiplying the D_{30} grain size of the formation by a factor of between 4 and 6 for coarser formation materials. Filter pack materials suitable for retaining formation materials in formations that are predominantly fine-grained are themselves, by necessity, relatively fine-grained (for example, fine to very fine sands), presenting several problems for well designers and installers. First, well screen slot sizes suitable for retaining such fine-grained filter pack materials are not widely available (the smallest commercially available slotted well casing is 6 slot, 0.006 in. [0.15 mm]; the smallest commercially available continuous-slot wire-wound screen is 4 slot, 0.004 in. [0.10 mm]). Second, the finest filter pack material practical for conventional (tremie tube) installation is a #40 by #70 sieve size, [425 to 212 µm] sand, which can be used with a well screen slot as small as 8 slot, 0.008-in., [0.20 mm]. Finer grained filter pack materials cannot be placed practically by either tremie tubes or pouring down the annular space or down augers.

(1) Pre-Packed Well Screen—The best method for ensuring proper installation of filter packs in predominantly fine-grained formation materials is to use pre-packed or sleeved screens, which are described in detail in Practice D6725. A #50 by #100 [300 to 150 µm] sieve size filter-pack sand can be used with a 6 slot size pre-packed or sleeved screen, and a #60 by #120 [250 to 125 µm] filter-pack sand can be used with a 4-slot slot size pre-packed or sleeved screen. Filter packs that are finer than these (for example, sands as fine as #100 by #120 [150 to 125 µm], or silica flour as fine as #200 mesh [75 µm]) can only be installed within stainless steel mesh sleeves that can be placed over pipe-based screens. While these sleeves, or the space between internal and external screens in a pre-packed well screen may be as thin as ½-in. [15 mm], the basis for mechanical retention dictates that a filter-pack thickness of only two or three grain diameters is needed to contain and control formation materials. Laboratory tests have demonstrated that a properly sized filter pack material with a thickness of less than ½-in. [15 mm] successfully retains formation particles regardless of the velocity of water passing through the filter pack. (**3, 4**)

(2) The theoretical limit of mechanical filtration for monitoring wells is defined by the finest filter pack material that can be practically installed via a pre-packed or sleeved screen encased within a very fine mesh screen of stainless steel or other suitable material. Dam filter design practice has found that a medium sand filter with sufficient fine fraction (10 to 30 %) of #50 to #100 sand with a D_{15} less than 0.2 mm is effective in retaining most all clay formation materials (**8, 9**).

NOTE 5—Although not recommended as standard practice, often a project requires drilling and installing the well in one phase of work. Therefore, the filter pack materials must be ordered and delivered to the drill site before soil samples can be collected. In these cases, the suggested well screen slot size and filter pack material combinations are presented in Table 1.

6.4 *Well Screen:*

6.4.1 *General*—Purposes of the well screen are to provide designed openings for groundwater flow through the well, and to prevent migration of filter pack and formation material into the well. Well screen design is based on either the grain-size distribution of the formation (in the case of a well with a naturally developed filter pack), or the grain-size distribution of the primary filter pack material (in the case of a filter-packed well). Screen openings must be small enough to retain most if not all of the formation or filter-pack materials, yet large enough to maintain groundwater flow velocities, from the well screen/filter pack interface back to the natural formation materials. Users are cautioned to limit entrance velocity as required to prevent turbulent flow conditions that result in mobilization of formation sediment and reduction in well efficiency especially when fine grained materials are present. These entrance flows are a result of the pumping rates during sampling and thus methods like low flow sampling (Practice D6771) are often employed for water quality sampling to minimize flow velocity and sample turbidity.

TABLE 1 Recommended (Achievable) Filter Pack Characteristics for Common Screen Slot Sizes

Size of Screen Opening, in. [mm]	Slot No.	Sand Pack Mesh Size Name(s)	1 % Passing Size (D_1), [mm]	Effective Size, (D_{10}), [mm]	30 % Passing Size (D_{30}), [mm]	Range of Uniformity Coefficient	Roundness (Powers Scale)
0.005 [0.125]	5[A]	100	[0.09 to 0.12]	[0.14 to 0.17]	[0.17 to 0.21]	1.3 to 2.0	2 to 5
0.010 [0.25]	10	20 to 40	[0.25 to 0.35]	[0.4 to 0.5]	[0.5 to 0.6]	1.1 to 1.6	3 to 5
0.020 [0.50]	20	10 to 20	[0.7 to 0.9]	[1.0 to 1.2]	[1.2 to 1.5]	1.1 to 1.6	3 to 6
0.030 [0.75]	30	10 to 20	[0.7 to 0.9]	[1.0 to 1.2]	[1.2 to 1.5]	1.1 to 1.6	3 to 6
0.040 [1.0]	40	8 to 12	[1.2 to 1.4]	[1.6 to 1.8]	[1.7 to 2.0]	1.1 to 1.6	4 to 6
0.060 [1.5]	60	6 to 9	[1.5 to 1.8]	[2.3 to 2.8]	[2.5 to 3.0]	1.1 to 1.7	4 to 6
0.080 [2.0]	80	4 to 8	[2.0 to 2.4]	[2.4 to 3.0]	[2.6 to 3.1]	1.1 to 1.7	4 to 6

[A] A 5-slot [0.152-mm] opening is not currently available in slotted PVC but is available in Vee wire PVC and Stainless; 6-slot opening may be substituted in these cases.

NOTE 6—This standard previously recommended limiting entrance velocities to less than 0.1 ft/s based on reference (**3**) to limit consequences of turbulent flow. Entrance velocities exceeding 0.10 ft/s [(0.03 m/s] do not always produce turbulent flow and mobilize formation sediment specially in coarse grained gravel packed aquifers where velocities as high as 1.5 ft/s can be stable with little reduction in well efficiency (**10**).

6.4.2 *Materials*—The well screen should be new, machine-slotted casing or continuous wrapped wire-wound screen composed of materials compatible with the monitoring environment, as determined by the site characterization program. The screen should be plugged at the bottom (unless a sediment sump is used), and the plug should generally be of the same material as the well screen. This assembly must have the capability to withstand well installation and development stresses without becoming dislodged or damaged. The length of the well screen open area should reflect the thickness of the target monitoring zone. Immediately prior to installation, the well screen should be cleaned (see either Practice D5088 or Practice D5608) with water from a source of known chemistry, if it is not certified clean by the manufacturer, and delivered, and maintained in a clean environment at the site.

NOTE 7—Well screens are most commonly composed of PVC or stainless steel. Stainless steel may be specified based on knowledge of the occurrence of microbially influenced corrosion in formations (specifically reducing or acid-producing conditions).

6.4.3 *Diameter*—The minimum nominal internal diameter of the well screen should be chosen based on factors specific to the particular application (such as the outside diameter of the purging and sampling device(s) to be used in the well). The typical rotary drilled well riser is a minimum of 2 in. [50 mm] diameter or larger. Well screens as small as ½-in. [15 mm] nominal diameter are available for use in monitoring well applications and can be used for special applications and are often used for smaller diameter direct push wells (Guide D6724 and Practice D6725).

6.4.4 *Design*—The design of the well screen should be determined based on the grain size analysis (in accordance with Test Method D422) of the interval to be monitored and the gradation of the primary filter pack material. In granular, non-cohesive formation materials that will fall in easily around the screen, filter packs can be developed from the native formation materials—filter pack materials foreign to the formation are not necessary. In these cases of naturally developed filter packs, the slot size of the well screen is determined using the grain size of the materials in the surrounding formation. The well screen slot size selected for this type of well completion should retain at least 70 % of formation materials—the finest 30 % of formation materials will be brought into the well during development, and the objectives of filter packing (to increase hydraulic conductivity immediately surrounding the well screen, and to promote easy flow of groundwater into and through the screen) will be met. In wells in which a filter pack material of a selected grain size distribution is introduced from the surface, the screen slot size selected should retain at least 90 %, and preferably 99 %, of the primary filter pack materials. The method for determining the primary filter pack design is described in 6.3.3.

6.4.5 *Prepacked or Sleeved Well Screens*—An alternative to designing and installing filter pack and well screens separately is to use a pre-packed or sleeved screen assembly. A pre-packed well screen consists of an internal well screen, an external screen or filter medium support structure, and the filter medium contained between the screens, which together comprise an integrated structure. The internal and external screens are constructed of materials compatible with the monitored environment, and are usually of a common slot size specified by the well designer to retain the filter pack material. The filter pack is normally an inert (for example, siliceous) granular material that has a grain-size distribution chosen to retain formation materials. A sleeved screen consists of a slotted pipe base over which a sleeve of stainless steel mesh filled with selected filter media is installed. Pre-packed or sleeved screens may be used for any formation conditions, but they are most often used where heaving, running or blowing sands make accurate placement of conventional well screens and filter packs difficult in rotary drilling, or where predominantly fine-grained formation materials are encountered. In the latter case, using pre-packed or sleeved screens is the only practical means of ensuring that filter pack materials of the selected grain-size distribution (generally fine to very fine sands) are installed to completely surround the screen (see 6.3.3.2).

NOTE 8—The practice of using a single well screen/filter pack combination (for example, 0.010 in. [0.25 mm]) well screen slot size with a 20/40 sand) for all wells, regardless of formation grain-size distribution, will result in siltation of the well and significant turbidity in samples when applied to formations finer than the recommended design. It will also result in the loss of filter pack, possible collapse of the screen, and invasion of overlying well construction materials (for example, secondary filter pack, annular seal materials, grout) when applied to formations coarser than the recommended design. For these reasons, the universal application of a single well screen/filter pack combination to all formations is not recommended, and should be avoided.

6.5 *Riser:*

6.5.1 *Materials*—The riser should be new pipe composed of materials that will not alter the quality of water samples for the constituents of concern and that will stand up to long-term exposure to the monitoring environment, including potential contaminants. The riser should have adequate wall thickness and coupling strength to withstand the stresses imposed on it during well installation and development (**3, 4**). Each section of riser should be certified as cleaned when new from the manufacturer or cleaned onsite (see either Practice D5088 or Practice D5608) using water from a source of known chemistry immediately prior to installation.

NOTE 9—Risers are generally constructed of polyvinyl chloride (PVC), galvanized steel, or stainless steel.

6.5.2 *Diameter*—The minimum nominal internal diameter of the riser should be chosen based on the particular application. Risers as small as ½-in. [15-20 mm] in diameter are available for applications in monitoring wells.

6.5.3 *Joints (Couplings)*—Threaded joints are recommended. Glued or solvent-welded joints of any type are not recommended because glues and solvents may alter the chemistry of water samples. Because square profile flush joint threads (Specification F480) are designed to be accompanied by O-ring seals at the joints, they do not require PTFE taping. However, tapered threaded joints should be PTFE taped to prevent leakage of water into the riser.

6.6 *Casing*—Where conditions warrant, the use of permanent casing installed to prevent or reduce communication between water-bearing zones is encouraged. The following subsections address both temporary and permanent casings.

6.6.1 *Materials*—The material type and minimum wall thickness of the casing should be adequate to withstand the forces of installation. All casing that is to remain as a permanent part of the installation (that is, in multi-cased wells) should be new and cleaned to be free of interior and exterior protective coatings.

NOTE 10—The exterior casing (temporary or permanent multi-cased) is generally composed of steel, although other appropriate materials may be used.

6.6.2 *Diameter*—Several different casing sizes may be required depending on the geologic formations penetrated. The diameter of the borehole and the well casing for conventionally filter packed wells should be selected so that a minimum annular space of 2 in. [50 mm] is maintained between the inside diameter of the casing and outside diameter of the riser to provide working space for a tremie pipe. For naturally developed wells and pre-packed or sleeved screen completions, this annular space requirement need not be met. In addition, the diameter of the casings in multi-cased wells should be selected so that a minimum annular space of 2 in. [50 mm] is maintained between the casing and the borehole (that is, a 2-in. [50 mm] diameter screen will require first setting a 6-in. [150 mm] diameter casing in a 10-in. [250 mm] diameter boring).

NOTE 11—Under difficult drilling conditions (collapsing soils, rock, or cobbles), it may be necessary to advance temporary casing. Under these conditions, a smaller annular space may be maintained.

6.6.3 *Joints (Couplings)*—The ends of each casing section should be either flush-threaded or beveled for welding.

6.7 *Sediment Sump*—A sediment sump, a length of blank pipe, generally of the same diameter and made of the same material as the riser and well screen—may be affixed to the bottom of the screen, and capped with a bottom plug, to collect fine-grained material brought into the well by the process of well development. A drainage hole may be drilled in the bottom of the sump to prevent the sump from retaining water in the event that the water level outside the well falls below the bottom of the well screen. Because the sediment that collects in the sump may harbor geochemistry-altering microflora and reactive metal oxides, this sediment must be removed periodically to minimize the potential for sample chemical alteration.

6.8 *Protective Casing:*

6.8.1 *Materials*—Protective casings may be made of aluminum, mild steel, galvanized steel, stainless steel, cast iron, or structural plastic pipe. The protective casing should have a lid capable of being locked shut by a locking device or mechanism.

6.8.2 *Diameter*—The inside dimensions of the protective casing should be a minimum of 2 in. [50 mm] and preferably 4 in. [100 mm] larger than the nominal diameter of the riser to facilitate the installation and operation of sampling equipment.

6.9 *Annular Sealants*—The materials used to seal the annulus may be prepared as a slurry or used un-mixed in a dry pellet, granular, or chip form. Sealants should be selected to be compatible with ambient geologic, hydrogeologic, geochemical and climatic conditions and any man-induced conditions (for example, subsurface contamination) anticipated during the life of the well. This standard was written assuming that thick liquid grout slurries are used for sealant from the filter zone to the surface as shown on Figs. 2 and 3. However, under certain conditions where bentonite pellets or chips can be placed and are allowed in the regulations and sampling plan, the grout zone shown on the figures may be backfilled with bentonite pellets or chips placed in lifts, hydrated and tamped, and placed in stages allowing zones to hydrate prior to placement of the next stage.

NOTE 12—An extensive research program on annular sealants has been conducted from 2001 through 2009 and subsequent years by the Nebraska Grout Task Force (**11**). This research included cement and bentonite grouts and also use of pellets and chips. The general findings of the study indicate all sealing methods suffer from some shrinkage in the portion of the well in the unsaturated zone. The best grouts were cement-sand, bentonite chips, neat cements, and bentonite slurries with more than 20 % solids. Especially problematic is the use of low solids content bentonite slurries leading to a ban on their use in California (**12**). The bentonite slurries used in this standard are high solids slurries with more than 20 % solids and bentonite slurry is not recommended in the unsaturated zone regardless of solids content.

6.9.1 *Bentonite*—Bentonite should be powdered, granular, pelletized, or chipped sodium montmorillonite from a commercial source, free of impurities that may adversely impact the water quality in the well. Pellets consist of roughly spherical units of moistened, compressed bentonite powder. Chips are irregularly shaped, and coarse granular units of bentonite free of additives. The diameter of pellets or chips selected for monitoring well construction should be less than one fifth the width of the annular space into which they are placed to reduce the potential for bridging. Granules consist of coarse to fine particles of unaltered bentonite, typically smaller than 0.2 in. [5.0 mm]. It is recommended that the water chemistry of the

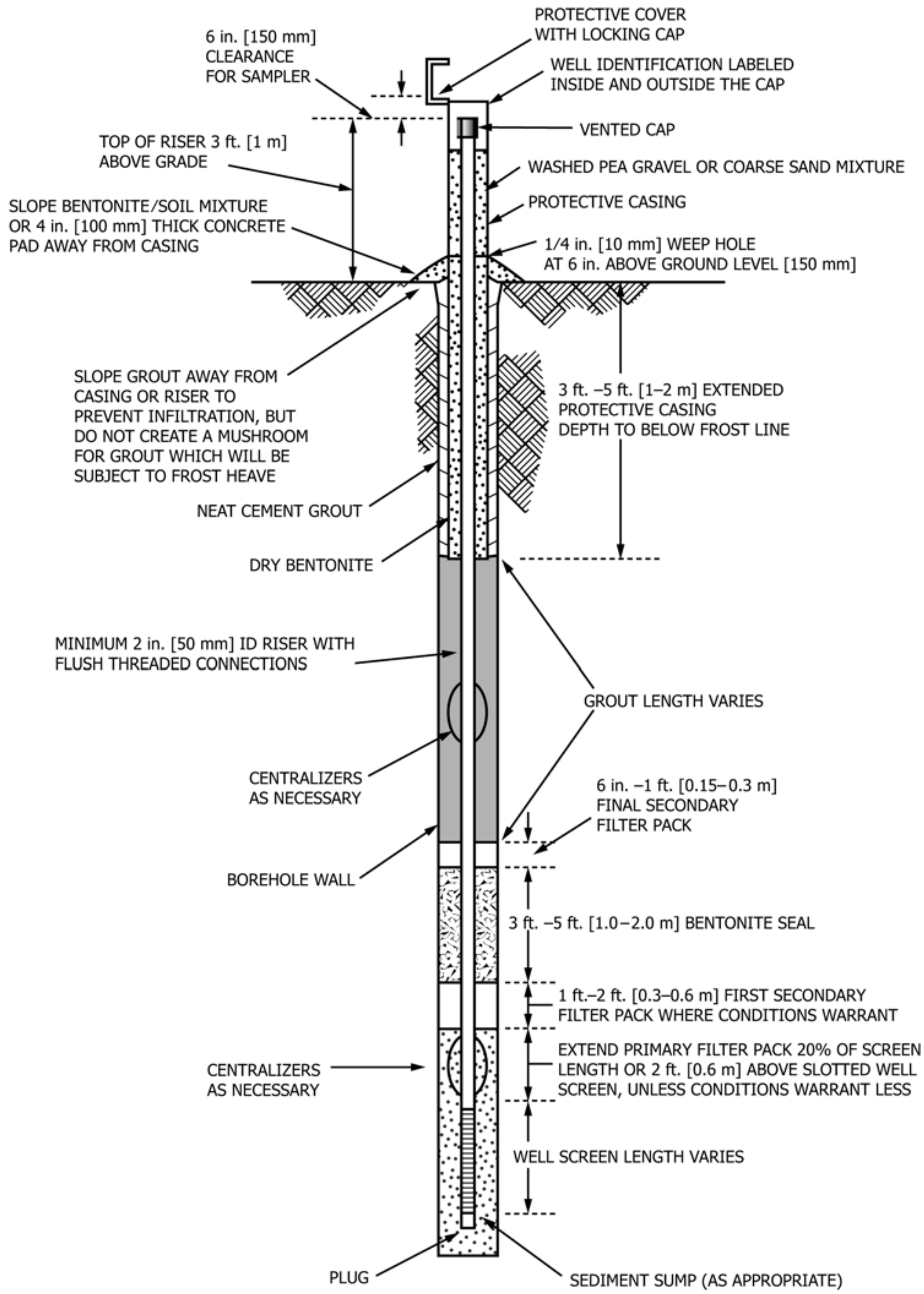


FIG. 2 Monitoring Well Design—Single-Cased Well

formation in which the bentonite is intended for installation be evaluated to ensure that it is suitable to hydrate the bentonite. Some water-quality conditions (for example, high chloride content, high concentrations of certain organic solvents or petroleum hydrocarbons) may inhibit the hydration of bentonite and result in an ineffective seal and in some cases, it may be desirable or advantageous to run field bench tests of sealants with actual source groundwater.

6.9.2 *Cement*—Each type of cement has slightly different characteristics that may be appropriate under various physical and chemical conditions. Cement should be one of the five Portland cement types that are specified in Specification C150. The use of quick-setting cements containing additives is not recommended for use in monitoring well installation. Additives may leach from the cement and influence the chemistry of water samples collected from the monitoring well. In high sulfate groundwater conditions (>0.5 %) sulfate resistant Type II cement should be considered for use. In critical cases where shrinkage is a concern with cement grouts, additives such as aluminum oxide or use of Type K cement can be used to give slightly expansive properties to the grout. Cement based grout has been shown to shrink away from PVC casing in the unsaturated zone (**11**). To prevent this, one can use swell additives, or use a steel riser pipe in that section of riser (**11**).

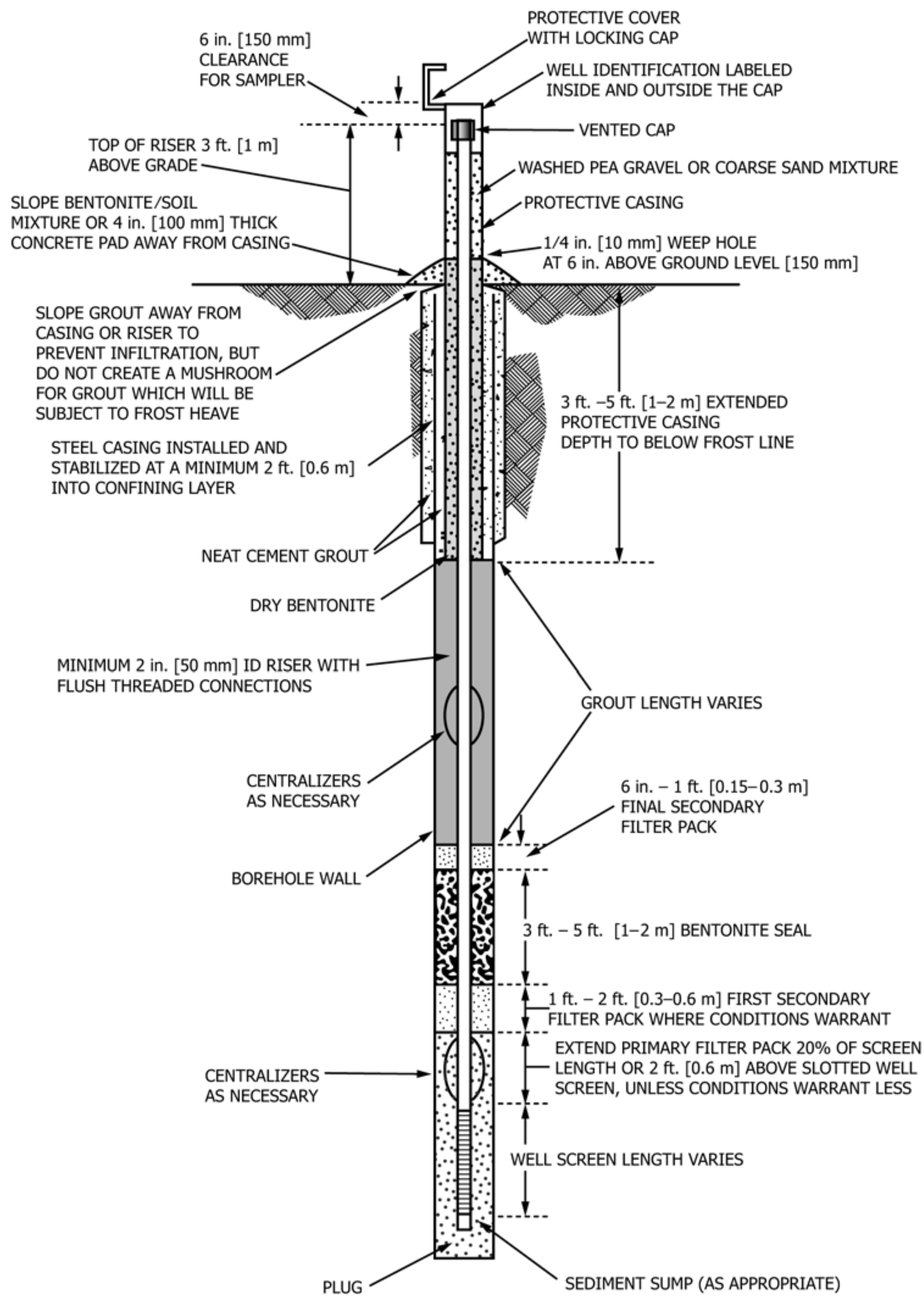


FIG. 3 Monitoring Well Design—Multi–Cased Well

Cement grout will generate heat during hydration that may jeopardize the integrity of the riser and in those cases the hole may require grouting in short lifts on deeper holes.

6.9.3 *Grout*—The grout backfill that is placed above the bentonite annular seal and secondary filters (see Fig. 2 and Fig. 3) is ordinarily a thick liquid slurry consisting of either a high solids bentonite (powder or granules, or both) base and water, or a Portland cement base and water with or without sand. Often, bentonite-based grouts are used when it is desired that the grout remain workable for extended periods of time during well construction or flexible (that is, to accommodate freeze-thaw cycles) during the life of the well. Cement-based grouts are often used when filling cracks in the surrounding geologic material, adherence to rock units, or a rigid setting is desired.

6.9.3.1 *Mixing*—The mixing (and placing) of a grout backfill should be performed with precisely recorded weights and volumes of materials, and according to procedures stipulated by the manufacturer that often include the order of component mixing. The grout should be thoroughly mixed with a paddle-type mechanical mixer or by recirculating the mix through a pump until all lumps are disintegrated. Lumpy grout should not be used in the construction of a monitoring well to prevent bridging within the tremie pipe.

NOTE 13—Lumps do not include lost circulation materials that may be added to the grout if excessive grout losses occur.

6.9.3.2 *Typical Bentonite-Based Grout*—A bentonite slurry containing at least 2 lb [0.9 kg] of bentonite for each gallon [3.8 L] per gallon of water for a minimum of 20 % solids (**13**). Bentonite should be placed in the water through a venturi device. Higher solids mixtures are difficult to mix and pump. Besides decreasing water to obtain high solids, one method is to use bentonite with inhibitors to delay hydration and another method is to add fine granular bentonite prior to pumping. Bentonite suppliers sell special premixed materials with pre-approved additives that are approved to yield 20 % solids requirement. Follow manufacturer's instructions and do not add excess water. Bentonite grouts should not be used for monitoring well annular sealants in the vadose zone of arid regions because of the possibility that they may desiccate. This could result in migration of water into the screened portion of the well from zones above the target monitoring zone.

(1) Bentonite chips and pellets may be a better alternate to achieving a seal below the water table, especially in shallow wells but depths of over 100 ft [30 m] can be achieved (**5, 11**). Pellets and chips of various sizes can be placed if there is a sufficient annulus, generally greater than 2 inches. Coated pellets or chips are used for greater depths. The levels should be checked and tamped and allowed to hydrate for a time period, as specified by the manufacturer or required in the work plan, for each increment backfilled.

NOTE 14—High solids bentonite grouts (minimum 20 % by weight with water) and other bentonite-based grouts may contain granular bentonite to increase the solids content and other components added under manufacturer's directions to either stiffen or retard stiffening of the mix. All additives to grouts should be evaluated for their effects on subsequent water samples.

6.9.3.3 *Typical Cement-Based Grout*—A typical cement-based grout consists of about 5 to 6 gal. of water per 94-lb [22 to 27 L of water per 50 kg bag] bag of Portland cement (equivalent to a water-cement-ratio of between 0.44 and 0.53). This is the same mixture used for C150 Neat Cement Slurry. Any water content exceeding this mixture will result in bleed water and excessive shrinkage and it is critical to check yield weights. Use Portland cement type I or II, or other type as required, and additives that may be required, by the work plan. Bentonite is often added after initial mixing from 3 to 8 % by dry weight to retard shrinkage, provide plasticity, and reduce pump wear. However, bentonite is chemically incompatible with cement and may reduce strength (**3, 4, and 5**). Calcium chloride additives have been used for early strength, yet it should be used cautiously because of heat generation (**5**).

(1) Sand-Cement Grout—Add about 1 ft^3 [0.03 m^3], about 100 lb [45 kg], of washed masonry sand to the mixture above. This grout is normally used for geothermal heat loop installation.

6.10 *Secondary Filter Packs (optional)*—These optional filter zones are sometimes used to protect primary filter pack and the bentonite seal from contamination. The secondary filter pack above the bentonite seal is strongly recommended if cement grout is to be placed above the bentonite seal to prevent infiltration if pellets/chips are not completely hydrated.

6.10.1 *Materials*—A secondary filter pack is a layer of material placed in the annulus between the primary filter pack and the bentonite seal, and/or between the bentonite seal and the grout backfill (see Fig. 2 and Fig. 3).

6.10.2 *Gradation*—The secondary filter pack should be uniformly graded fine sand with 100 % by weight passing the #30 standard sieve, and less than 2 % by weight passing the #200 standard sieve.

6.11 *Annular Seal and Filter Pack Installation Equipment*—The equipment used to install the annular seals and filter pack materials should be cleaned (if appropriate for the selected material) using water from a source of known quality prior to use. This procedure is performed to prevent the introduction of materials that may ultimately alter water quality samples.

7. Drilling Methods

7.1 The type of equipment required to create a stable, open, vertical borehole for installation of a monitoring well depends upon the site geology, hydrology, and the intended use of the data. Engineering and geological judgment and some knowledge of subsurface geological conditions at the site is required for the selection of the appropriate drilling method(s) utilized for drilling the exploratory boreholes and monitoring wells (see Guide D6286). Appropriate drilling methods for investigating and installing monitoring wells at a site may include any one or a combination of several of the following methods: hollow-stem auger (Guide D5784); sonic drilling (Practice D6914), direct (mud) rotary (Guide D5783); direct air-rotary (Guide D5782); direct rotary wireline casing advancement (Guide D5876); dual-wall reverse-circulation rotary (Guide D5781); cable-tool (Guide D5875); or various casing advancement methods (Guide D5872). Whenever feasible, it is advisable to utilize drilling procedures that do not require the introduction of water or drilling fluids into the borehole, and that optimize cuttings control at ground surface. Where the use of water or drilling fluid is unavoidable, the selected fluid should have as little impact as possible on the water samples for the constituents of interest. The chemistry of the fluid to be used should be evaluated to determine the potential for water quality sample alteration. In addition, care should be taken to remove as much drilling fluid as possible from the well and the surrounding formation during the well development process. It is recommended that if an air compressor is used, it be equipped with an oil air filter or oil trap to minimize the potential for chemical alteration of groundwater samples collected after the well is installed.

8. Monitoring Well Installation

8.1 *Stable Borehole*—A stable borehole must be constructed prior to attempting the installation of monitoring well screen and riser. Steps must be taken to stabilize the borehole before attempting installation if the borehole tends to cave or blow in, or both. Boreholes that are not straight/plumb or are partially obstructed should be corrected prior to attempting the installation procedures described herein.

8.2 *Assembly of Well Screen and Riser:*

8.2.1 *Handling*—The well screen, sediment sump, bottom plug and riser should be either certified clean from the manufacturer or steam-cleaned or high-pressure hot-water washed (whichever is appropriate for the selected material) using water from a source of known chemistry immediately prior to assembly. Personnel should take precautions to assure that grease, oil, or other contaminants that may ultimately alter the water sample do not contact any portion of the well screen and riser assembly. As one precaution, for example, personnel should wear a clean pair of cotton, nitrile or powder-free PVC (or equivalent) gloves while handling the assembly.

8.2.2 *Riser Joints (Couplings)*—Flush joint risers with square profile (Specification F480) threads do not require PTFE taping to achieve a water tight seal; these joints should not be taped. O-rings made of a material of known chemistry, selected on the basis of compatibility with contaminants of concern and prevailing environmental conditions, should be used to assure a tight seal of flush-joint couplings. Couplings are often tightened by hand; however, if necessary, steam-cleaned or high-pressure water-cleaned wrenches may be utilized. Precautions should be taken to prevent damage to the threaded joints during installation, as such damage may promote leakage past the threads.

8.3 *Setting the Well Screen and Riser Assembly*—When the well screen and riser assembly is lowered to the predetermined level in the borehole and held in position, the assembly may require ballast to counteract the tendency to float in the borehole. Ballasting may be accomplished by filling the riser with water from a source of known and acceptable chemistry or, preferably, using water that was previously removed from the borehole. Alternatively, the riser may be slowly pushed into the fluid in the borehole with the aid of hydraulic rams on the drill rig and held in place as additional sections of riser are added to the column. Care must be taken to secure the riser assembly so that personnel safety is assured during the installation. The assembly must be installed straight and plumb, with centralizers installed at appropriate locations (typically every 20 to 30 ft [5 to 10 m]). Difficulty in maintaining a straight installation may be encountered where the weight of the well screen and riser assembly is significantly less than the buoyant force of the fluid in the borehole. The riser should extend above grade and be capped temporarily to deter entrance of foreign materials during final completion.

8.4 *Installation of the Primary Filter Pack:*

8.4.1 *Volume of Filter Pack*—The volume of filter pack required to fill the annular space between the well screen and borehole should be calculated, measured, and recorded on the well completion diagram during installation. To be effective, the filter pack should extend above the screen for a distance of about 20 % of the length of the well screen but not less than 2 ft [0.5-1 m] (see Figs. 2 and 3). Where there is hydraulic connection between the zone to be monitored and the overlying strata, this upward extension should be gauged to prevent seepage from overlying hydrologic units into the filter pack. Seepage from other units may alter hydraulic head measurements or the chemistry of water samples collected from the well.

8.4.2 *Placement of Primary Filter Pack*—Placement of the well screen is preceded by placing no less than 2 % and no more than 10 % of the primary filter pack into the bottom of the borehole using a decontaminated, flush threaded, 1-in. [25-mm] minimum internal diameter tremie pipe. Alternatively, the filter pack may be added directly between the riser pipe and the auger or drive/temporary casing and the top of the filter pack located using a tamper or a weighted line. The well screen and riser assembly is then centered in the borehole. This can be done using one or more centralizer(s) or alternative centering devices located not more than 10 ft [3 m] above the bottom of the well screen (see Figs. 2 and 3). Centralizers should not be located in the well screen. The remaining primary filter pack is then placed in increments as the tremie is gradually raised or as the auger or drive/temporary casing is removed from the borehole. As primary filter pack material is poured into the tremie pipe, water from a source of known and acceptable chemistry may be added to help deliver the filter pack to the intended interval in the borehole. Pre-wetting sand may aid in placement. The tremie pipe or a weighed line can be used to measure the top of the primary filter pack as work progresses. Place the sand in lifts and surge (see 9.3) or vibrate casing to aid in settlement, if necessary, aid in proper placement. If bridging of the primary filter pack material occurs, the bridged material should be broken mechanically prior to proceeding with the addition of more filter pack material. The elevation (or depth below ground surface), volume, and gradation of primary filter pack should be recorded on the well completion diagram (see Appendix X1 for examples).

8.4.3 *Withdrawal of the Temporary Casing/Augers*—If used, the drive/temporary casing or hollow stem auger is withdrawn, usually in stipulated increments. Care should be taken to avoid lifting the riser with the withdrawal of the temporary casing/augers. To limit borehole collapse in stable formations, the temporary casing or hollow stem auger is usually withdrawn until the lower-most point on the temporary casing or hollow stem auger is at least 2 ft [0.6 m], but no more than 5 ft [1.5 m] above the filter pack for unconsolidated materials; or at least 5 ft [1.5 m], but no more than 10 ft [3.0 m], for consolidated materials. In highly unstable formations, withdrawal intervals may be much less. After each increment, it should be ascertained that the primary filter pack has not been displaced during the withdrawal operation (using a weighed measuring device).

8.5 *Placement of First Secondary Filter*—A secondary filter pack may be installed above the primary filter pack to prevent the intrusion of the bentonite grout seal into the primary filter pack (see Figs. 2 and 3). To be effective, a measured and recorded volume of secondary filter material should be added to extend 1 to 2 ft [0.3 to 0.6 m] above the primary filter pack. As with the primary filter, a secondary filter must not extend into an overlying hydrologic unit (see 8.4.1). The well designer should evaluate the need for this filter pack by considering the gradation of the primary filter pack, the hydraulic heads between adjacent units, and the potential for grout intrusion into the primary filter pack. The secondary filter material is poured into the annular space through a decontaminated, flush threaded, 1-in. [25-mm] minimum internal diameter tremie pipe lowered to within 3 ft [1.0 m] of the placement interval.

Water from a source of known and acceptable chemistry may be added to help deliver the filter pack to its intended location. The tremie pipe or a weighed line can be used to measure the top of the secondary filter pack as work progresses. The elevation (or depth below ground surface), volume, and gradation of the secondary filter pack should be recorded on the well completion diagram (see Appendix X1).

8.6 *Installation of the Bentonite Seal*—A bentonite pellet or a slurry seal is placed in the annulus between the borehole and the riser pipe on top of the secondary or primary filter pack (see Figs. 2 and 3). This seal retards the movement of cement-based grout backfill into the primary or secondary filter packs. To be effective, the bentonite seal should extend above the filter packs approximately 3 to 5 ft [1.0 to 1.5 m], depending on local conditions. The bentonite slurry seal should be installed using a positive displacement pump and a side-discharge tremie pipe lowered to the top of the filter pack. The tremie pipe should be raised slowly as the bentonite slurry fills the annular space. Bentonite pellets or chips may be poured from the surface and allowed to free-fall into the borehole. As a bentonite pellet or chip seal is poured into the borehole, a tamper may be necessary to tamp pellets or chips into place or to break bridges formed as the pellets or chips stick to the riser or the walls of the water-filled portion of the borehole. If the bentonite seal is installed above the water level in the borehole, granular or chip bentonite should be used as the seal material. Granular bentonite should be poured into the borehole and installed in lifts of 2 in. [50 mm], then hydrated with water from a source of known chemistry. The tremie pipe or a weighed line can be used to measure the top of the bentonite seal as the work progresses. Sufficient time should be allowed for the bentonite pellet seal to hydrate or the slurry annular seal to expand prior to grouting the remaining annulus. The volume and elevation (or depth below ground surface) of the bentonite seal material should be measured and recorded on the well completion diagram (see Appendix X1).

8.7 *Final Secondary Filter Pack (optional)*—A 6-in. to 1-ft [0.15 to 0.3-m] secondary filter may be placed above the bentonite seal in the same manner described in 8.5 (see Figs. 2 and 3). This secondary filter pack will provide a layer over the bentonite seal to limit the downward movement of cement-based grout backfill into the bentonite seal. The volume, elevation (or depth below ground surface), and gradation of this final secondary filter pack should be documented on the well completion diagram (see Appendix X1).

8.8 *Grouting the Annular Space*—As noted in 6.9, the annular sealant described below is for grouts which are pumped into the annulus. In some cases the annular seal can be constructed using bentonite chips or pellets.

8.8.1 *General*—Grouting procedures vary with the type of well design. The following procedures will apply to both single- and multi-cased monitoring wells. Paragraphs 8.8.2 and 8.8.3 detail those procedures unique to single- and multi-cased installations, respectively.

8.8.1.1 *Volume of Grout*—An ample volume of grout should be mixed on site to compensate for unexpected losses to the formation. The use of alternate grout materials, including grout containing gravel, may be necessary to control zones of high grout loss. The volume and location of grout used to backfill the remaining annular space is recorded on the well completion diagram (see Appendix X1).

8.8.1.2 *Grout Installation Procedures*—The grout should be pumped down hole through a side-discharge tremie pipe using a positive displacement pump (for example, a diaphragm pump, moyno pump, or similar pump) to reduce the chance of leaving voids in the grout, and to displace any liquids and drill cuttings that may remain in the annulus. In very shallow wells, grouting may be accomplished by gravity feeding grout through a tremie pipe. With either method, grout should be introduced in one continuous operation until full-strength grout flows out of the borehole at the ground surface without evidence of drill cuttings, drilling fluid, or water.

8.8.1.3 *Grout Setting and Curing*—The riser should not be disturbed until the grout sets and cures for the amount of time necessary to prevent a break in the seal between the grout and riser. The amount of time required for the grout to set or cure will vary with the grout mix and ambient temperature and should be documented on the well completion diagram (see Appendix X1).

8.8.2 *Specific Procedures for Single-Cased Wells*—Grouting should begin at a level directly above the final secondary filter pack (see Fig. 2) if used, or above the bentonite pellet, chip or slurry seal. Grout should be pumped using a side-discharge tremie pipe to dissipate the fluid-pumping energy against the borehole wall and riser, reducing the potential for infiltration of grout into the primary filter pack. The tremie pipe should be kept full of grout from start to finish, with the discharge end of the pipe completely submerged as it is slowly and continuously lifted. Approximately 5 to 10 ft [1.5 to 3.0 m] of tremie pipe should remain submerged until grouting is complete. For deep installations or where the joints or couplings of the selected riser cannot withstand the collapse stress exerted by a full column of grout as it is installed, a staged grouting procedure may be used. If used, the drive/temporary casing or hollow-stem auger should be removed in increments immediately following each increment of grout installation and before the grout begins to set. If casing removal does not commence until grout pumping is completed, then, after the casing is removed, additional grout may be periodically pumped into the annular space to maintain a continuous column of grout up to the ground surface.

8.8.3 *Specific Procedures for Multi-Cased Wells*—If the outer casing of a multi-cased well cannot be driven to form a tight seal between the surrounding stratum (strata) and the casing, it should be installed in a pre-drilled borehole. After the borehole has penetrated not less than 2 ft. [0.6 m] of the first targeted confining stratum, the outer casing should be lowered to the bottom of the boring and the annular space pressure grouted. Pressure grouting requires the use of a grout shoe or packer installed at the end of the outer casing to prevent grout from moving up into the casing. The grout must be allowed to cure and form a seal between the casing and the borehole prior to advancing the hole to the next hydrologic unit. This procedure is repeated as necessary to advance the borehole to the desired depth. Upon reaching the final depth, the riser and screen should be set through the inner casing. After placement of the filter packs and bentonite seal, the remaining annular space is grouted as described in 8.8.2 (see Fig. 3).

NOTE 15—When using a packer, pressure may build up during grout injection and force grout up the sides of the packer and into the casing.

8.9 *Well Protection*—Well protection refers specifically to installations made at the ground surface to deter unauthorized entry to the monitoring well, to prevent damage to or destruction of the well, and to prevent surface water from entering the annulus. The methods described in Practice D5787 should be used for well protection.

8.9.1 *Protective Casing*—Protective casing should be used for all monitoring well installations in areas that may be subject to frost heaving, the protective casing should extend from below the depth of frost penetration (typically 3 to 5 ft [1.0 to 1.5 m] below grade or more, depending on local conditions), to slightly above the top of the well casing. The protective casing should be initially placed before final set of the grout. The protective casing should be sealed and immobilized in concrete placed around the outside of the protective casing above the set grout. The protective casing should be stabilized in a position concentric with the riser (see Figs. 2 and 3). Sufficient clearance, usually 6 in. [150 mm] should be maintained between the lid of the protective casing and the top of the riser to accommodate sampling equipment. A ¼-in. [6-mm] diameter weep hole should be drilled in the protective casing approximately 6 in. [150 mm] above ground surface to permit water to drain out of the annular space between the protective casing and the riser. Multiple weep holes can be used at variable heights to accommodate any heave of bentonite. In cold climates, this hole will also prevent water freezing between the protective casing and the well casing. Dry bentonite pellets, granules, or chips should then be placed in the annular space below ground level within the protective casing. Coarse sand or pea gravel or both should be placed in the annular space above the dry bentonite pellets and to just above the weep hole to prevent entry of insects. All materials chosen should be documented on the well completion diagram (Appendix X1). The monitoring well identification number should be clearly visible on the inside and outside of the protective casing. In areas where protective casing is not an option, flush mounted well vaults should be considered.

8.9.2 *Completion of Surface Installation*—The well protection installation may be completed in one of three ways:

8.9.2.1 In areas subject to frost heave, place a soil or bentonite/sand layer adjacent to the protective casing sloped to direct water drainage away from the well.

8.9.2.2 In regions not subject to frost heave, a concrete pad, sloped slightly to provide water drainage away from the well, should be placed around the installation. An option to a concrete pad is a 6 in. [150 mm] gravel pad placed atop a geotextile and this design may be advantageous in areas subject to frost heave.

8.9.2.3 Where monitoring well protection must be installed flush with the ground, an internal cap should be fitted on top of the riser within the manhole or vault. This cap should be leak-proof so that if the vault or manhole should fill with water, the water will not enter the well casing. Ideally, the manhole cover cap should also be leak-proof.

8.9.3 *Additional Protection*—In areas where there is a high probability of damaging the well (high traffic, heavy equipment, poor visibility), it may be necessary to enhance the normal protection of the monitoring well through the use of posts, markers, signs, or other means, as described in Practice D5787. Although installation in traffic areas should be avoided, if the well is installed in an area subject to vehicular traffic, or similar loading, then the well box must be traffic-rated and flush mounted or recessed (Note 16). The level of protection should meet the damage threat posed by the location of the well.

NOTE 16—Many manufacturers offer well boxes, vaults, and manholes that are traffic-rated in compliance with ASSHTO loading requirements such as HS-20, HS-40, and M306 loadings. For those wells installed in snow plowing areas, the additional protection should include use of a traffic-rated well box that is recessed slightly below grade. A concrete pad around the well box should gently slope from the original grade to the recessed well box.

9. Well Development

9.1 *General*—Well development serves to remove fine-grained material from the well screen and filter pack that may otherwise interfere with water quality analyses, to restore the formation properties disturbed during the drilling process, and to improve the hydraulic characteristics of the filter pack and hydraulic communication between the well and the hydrologic unit adjacent to the well screen. Methods of well development vary with the physical characteristics of hydrologic units in which the monitoring well is screened and with the drilling method used.

9.2 *Development Methods and Procedures*—The methods and procedures for well development described in Guide D5521 should be followed to ensure a proper well completion.

9.3 *Timing and Duration of Well Development*—Well development should begin either after the riser, well screen and filter pack are installed and before the bentonite seal and grout are installed (the preferred time), or after the monitoring well is completely installed and the grout has cured or set. In the first case, the installer may add filter pack material to the borehole before the bentonite seal is installed to compensate for settlement that typically occurs during the development process. This allows the installer to maintain the desired separation between the top of the screen and the bentonite seal. In the latter case, the possibility exists that settlement of the filter pack may result in the bentonite seal settling into the top of the screen. Development should be continued until representative water, free of the drilling fluids, cuttings, or other materials introduced or produced during well construction, is obtained. Representative water is assumed to have been obtained when turbidity readings stabilize and the water is visually clear of suspended solids. The minimum duration of well development will vary with the method used to develop the well. The timing and duration of well development and the turbidity measurements should be recorded on the well completion diagram (see Appendix X1).

NOTE 17—Wells in fine grained formations of low hydraulic conductivity (aquitards or confining units) can be damaged using well development techniques for coarse grained aquifers. A simple method for developing would be to use two pumps, one to inject water at the base and the other to remove the sediments. Emptying wells in fine grained soils can damage the surrounding formation.

9.4 *Well Recovery Test*—A well recovery test may be performed immediately after and in conjunction with well development. The well recovery test provides an indication of well performance and provides data for estimating the hydraulic conductivity of the screened hydrologic unit. Readings should be taken at intervals suggested in Table 2 until the well has recovered to 90 % of its static water level.

NOTE 18—If a monitoring well does not recover sufficiently for sampling within a 24-h period and the well has been properly developed, the installation should not generally be used as a monitoring well for detecting or assessing low level organic constituents or trace metals. The installation may, however, be used for long-term water-level monitoring if measurements of short-frequency water-level changes are not required.

NOTE 19—Slug tests for the purposes of determining formation hydraulic conductivity in installed wells may be adversely affected by the well installation zone filter pack and slot size. Wells with 10 slot screen may underestimate formation K in high K soils. For more information on slug testing monitoring wells refer to Ref. (**14**).

10. Installation Survey

10.1 *General*—The vertical and horizontal position of each monitoring well in the monitoring system should be surveyed and subsequently mapped by a licensed surveyor. The well location map should include the location of all monitoring wells in the system and their respective identification numbers, elevations of the top of riser position to be used as the reference point for water-level measurements, and the elevations of the ground surface protective installations. The locations and elevations of all permanent benchmark(s) and pertinent boundary marker(s) located on-site or used in the survey should also be noted on the map.

10.2 *Water-Level Measurement Reference*—The water-level measurement reference point should be permanently marked, for example, by cutting a V-notch into the top edge of the riser pipe. This reference point should be surveyed in reference to a national vertical datum, such as NAVD83.

TABLE 2 Suggested Recording Intervals for Well Recovery Tests

Time Since Starting Test	Time Interval
0 to 15 min	1 min
15 to 50 min	5 min
50 to 100 min	10 min
100 to 300 min (5 h)	30 min
300 to 1440 min (24 h)	60 min

10.3 *Location Coordinates*—The horizontal location of all monitoring wells (active or decommissioned) should be surveyed by reference to a standardized survey grid or by boundary survey.

10.4 *Borehole Deviation Survey*—A borehole deviation survey, to determine the direction and distance of the bottom of the well relative to the top of the well and points in between, should be completed in wells deeper than 100 feet [30 m] and in wells installed in dipping formations.

11. Report: Test Data Sheet(s)/Form(s)

11.1 The methodology used to specify how data are recorded on the test data sheet(s)/form(s), as given below, is covered in 1.6.

11.2 Record as a minimum the following general information:

11.2.1 To demonstrate that the goals set forth in the scope have been met, a monitoring well network report should be prepared. This report should:

11.2.1.1 Locate the area investigated in terms pertinent to the project. This should include sketch maps or aerial photos on which the exploratory boreholes, piezometers, sample areas, and monitoring wells are located, as well as topographic items relevant to the determination of the various soil and rock types, such as contours, streambeds, etc. Where feasible, include a geologic map and geologic cross sections of the area being investigated.

11.2.1.2 Include copies of all well boring test pits and exploratory borehole logs (example, Appendix X1 (Fig. X1.5)), initial and post-completion water levels, all laboratory test results, and all well completion diagrams (example, Appendix X1 (Figs. X1.1-X1.4)).

11.2.1.3 Include the well installation survey.

11.2.1.4 Describe and relate the findings obtained in the initial reconnaissance and field exploration (Section 5) to the design and installation procedures selected (Sections 7 – 9) and the surveyed locations (Section 10).

11.2.1.5 This report should include a recommended decommissioning procedure that is consistent with those described in Guide D5299 and/or with applicable regulatory requirements.

11.3 Record as a minimum the following data:

11.3.1 Record all drilling depths, and installation depths, the nearest 0.1 ft. [0.03 m] or better on well completion diagrams used in 11.3.

11.3.2 Record all quantities and volumes for filter pack, grout, and backfill to two significant digits or better.

12. Keywords

12.1 aquifer; borehole drilling; geophysical exploration; groundwater; monitoring well; site investigation

APPENDIX

(Nonmandatory Information)

X1. EXAMPLE WELL COMPLETION FORMS

X1.1 See Figs. X1.1-X1.5.

Agency **Well Completion Report**

Site Number: ____________ County: ____________

Site Name: ____________ Well #: ____________

State Plane Coordinate: X _____ Y _____ (or) Latitude: ___° ___′ ___″ Longitude: ___° ___′ ___″ Borehole #: ____________

Surveyed by: ____________ IL Registration #: ____________

Drilling Contractor: ____________ Driller: ____________

Consulting Firm: ____________ Geologist: ____________

Drilling Method: ____________ Drilling Fluid (Type): ____________

Logged By: ____________ Date Started: ____________ Date Finished: ____________

Report Form Completed By: ____________ Date: ____________

ANNULAR SPACE DETAILS

Elevations (MSL)* **Depths (BGS)** **(.01 ft.)**

Type of Surface Seal: ____________

Type of Annular Sealant: ____________

Installation Method: ____________

Setting Time: ____________

Type of Bentonite Seal - - Granular, Pellet, Slurry (Choose One)

Installation Method: ____________

Setting Time: ____________

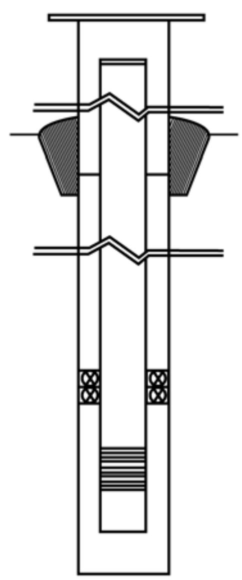
Type of Sand Pack: ____________

Grain Size: ________ (Sieve Size)

Installation Method: ____________

Type of Backfill Material: ____________ (if applicable)

Installation Method: ____________



Elevations (MSL)*	Depths (BGS)	(.01 ft.)
_____	_____	Top of Protective Casing
_____	_____	Top of Riser Pipe
_____	_____	Ground Surface
_____	_____	Top of Annular Sealant
_____	_____	Static Water Level (After Completion)
_____	_____	Top of Seal
_____	_____	Top of Sand Pack
_____	_____	Top of Screen
_____	_____	Bottom of Screen
_____	_____	Bottom of Well
_____	_____	Bottom of Borehole

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIAL

(Choose one type of material for each area)

Protective Casing	SS304, SS316, PTFE, PVC, or Other
Riser Pipe Above W.T.	SS304, SS316, PTFE, PVC, or Other
Riser Pipe Below W.T.	SS304, SS316, PTFE, PVC, or Other
Screen	SS304, SS316, PTFE, PVC, or Other

CASING MEASUREMENTS

Diameter of Borehole (inches)	
ID of Riser Pipe (inches)	
Protective Casing Length (feet)	
Riser Pipe Length(feet)	
Bottom of Screen to End Cap (feet)	
Screen Length (1st slot to last slot) (feet)	
Total Length of Casing (feet)	
Screen Slot Size **	

** Hand-Slotted Well Screens are Unacceptable

FIG. X1.1 Well Completion Report

MONITORING WELL COMPLETION REPORT

PART I: GENERAL INFORMATION

Well ID:	Site Name:		Well Install Date
Facility ID	Alternate ID	FLUWID #	WMD Permit #
Well Purpose ☐ Background ☐ Intermediate ☐ Compliance ☐ Other (explain)			
Latitude (to nearest 0.1 seconds)		Longitude (to nearest 0.1 seconds)	
Latitude and Longitude collection method: ☐ DGPS ☐ AGPS ☐ MAP ☐ ZIPCODE ☐ DPHO ☐ UNKNOWN ☐ OTHER			

PART II: WELL CONSTRUCTION DETAILS

Water Well Contractor Name		Contractor License #
Company Name		
Construction Method: ☐ Hollow Stem Auger ☐ Solid Stem Auger ☐ Water/Mud Rotary ☐ Air Rotary ☐ Cable Tool ☐ Direct Push ☐ Sonic ☐ Other (describe)		Aquifer Monitored
Top of Casing Elevation (NVGD or NAVD)	Ground Surface Elevation (NVGD or NAVD)	

Casing

Material	Inside Diameter	Outside Diameter	Depth (ft.)	
			From	To

Screen

Material	Inside Diameter	Outside Diameter	Depth (ft.)		Slot Size
			From	To	

Annulus

Material including additives for sealant	Size of Material	Amount (# of bags)	Depth (ft.)		Installation Method
			From	To	

FIG. X1.2 Monitoring Well Completion Report

PART III: WELL DEVELOPMENT DETAILS

Well Development Date	Well Development Method: ☐ Surge/Pump ☐ Pump ☐ Compressed Air ☐ Other (explain)		
Development Duration			
Pumping Rate	Maximum Drawdown	Well Purged Dry ☐ yes ☐ no	Pumping Condition ☐ continuous ☐ intermittent
Turbidity (if Measured):			Stabilized Water Level (BLS)
Start:	End:		
Water appearance (color and odor) at start of development:			
Water appearance (color and odor) at end of development:			

Report Prepared By:	Date
Title/Company	License #

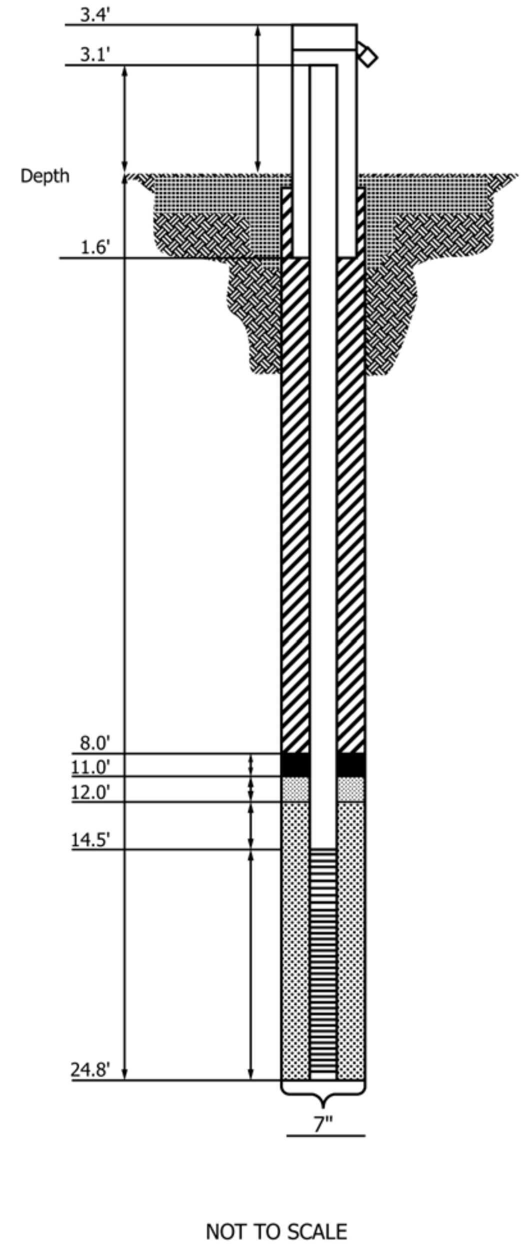
PLEASE ATTACH BORING LOG

Remarks

FIG. X1.3 Monitoring Well Completion Report (*continued*)

Well No.	Installation:	
Project No.	Client/Project:	Site:

Contractor:	Drill Contractor:
Start Date: 7/16/04 End Date: 7/16/04	Elevation (Top of Pad): 422.7048 ft amsi
	Coordinates: N 863681.1098
	E 827465.7528



Protective Casing
Material Type: Aluminum
Diameter: 4 inch square
Depth BGS: 1.6 ft Weep Hole? (Y/N): Yes
Guard Posts (Y/N): Y
No: Type:

Surface Pad
Composition and Size: 2 ft × 2ft Concrete

Riser Pipe
Type: SCH 80 PVC
Diameter: 2 inch
Total Depth (Ground surface to TOS): 14.5'
Ventilated Cap (Y/N): No

Grout
Composition & Proportions: 50# Bentonite Grout/24 gal water
Grout Type: Wyo-Ben Grout Well
Weight (lbs/gal): N/A
% Solids: 20%
Tremied (Y/N): Yes
Interval BGS: 0.0' – 8.0'

Centralizers (Y/N): No
Depth(s): N/A

Bentonite Seal
Type: ¼ in. bentonite pellets
Source: Pel-Plug
Setup/Hydration Time: 1.5 hour Vol. Fluid Added: 5 gal
Tremied (Y/N): No

Secondary Filter Pack
Graduation Designation: 100 mesh
Grain Size: N/A
Type: Colorado Silica Sand
Amt Used: 50#
Tremied (Y/N): Yes

Primary Filter Pack
Graduation Designation: 10/20 mesh
Grain Size: N/A
Type: FilterSil
Amt. Used: 150#
Tremied (Y/N): Yes

Screen
Type: SCH 80 PVC
Diameter: 2 in.
Length: 10 ft
Slot Size and Type: 0.010 slotted
Interval BGS: 14.5' – 24.5'

Isolation Casing (Y/N): No
Type:
Diameter:
Total Length:

Backfill Plug: N/A
Material:
Setup/Hydration Time:
Tremied (Y/N):

Monitoring Well Construction Log

FIG. X1.4 Monitoring Well Construction Log

Field Boring Log

Page ___ of ___

Agency

Site ID No. ________________ Federal ID No. ________________

Site Name: ________________________________

Quadrangle: ________________ Sec. ________ T. ______ R. ________

UTM (or State Plane) Coord. N. (X) ________________ E. (Y) ________________

Latitude: ____° ____′ ____″ Longitude: ____° ____′ ____″

Boring Location: ________________________________

Drilling Equipment: ________________________________

County: ________________________

Boring No. __________ Monitoring Well No. __________

Surface Elevation: ________ Completion Depth: ________

Auger Depth: __________ Rotary Depth: __________

Date: Start: __________ Finish: __________

Elev.	Description of Material	Graphic Log	Depth In Feet	SAMPLES						Personnel: G - D - H - H -
				Sample No.	Sample Type	Sample Recovery (X)	Penetrometer	N Values (Blow Counts)	OVA or HNU Readings	REMARKS

FIG. X1.5 Field Boring Log

REFERENCES

(1) ANSI/NGWA-01-14 Water Well Construction Standard (2014), National Ground Water Association, NGWA Press, 601 Dempsey Rd., Westerville, OH 43081, www.ngwa.org , May.

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SUMMARY OF CHANGES

In accordance with Committee D18 policy, this section identifies the location of changes to this standard since the last edition (2004(Reapproved 2010)$^{\varepsilon 1}$) that may impact the use of this standard.

(1) Revised the scope to include finer grained formations and addressing turbidity and new sampling methods including low flow and passive sampling methods. Added limitations in rock.
(2) Units were converted to English [SI] rationalized units added significant digits requirements. Figures 2 and 3 were revised to show rationalized units.
(3) Editorial adjustments and major revisions to the terminology section to comply with D-18 requirements and standard definitions in D653. Many terms from D5092 were adopted into D653 previous to this revision.
(4) Referred to NGWA national standard and State of California standards for groundwater monitoring wells.
(5) Improved Figure 1 to show how to size filter pack and select slot size example from Nielsen.
(6) Clarified 2004 revisions for filter design for fine grained formation in section 6.3.3.
(7) Revised well screen entrance velocity in 6.4 to allow for fast flow in coarse grained stable formations.
(8) Revised borehole sealing requirements based on new research from Nebraska Grout Task force. Low solids bentonite slurries are not allowed and provisions were made for use of 20 % minimum solids and use of bentonite chips and pellets in section 6.9.
(9) Modified grout mixture information for both bentonite and cement based grouts in 6.9.3
(10) Changed report section to comply with D18 requirements.
(11) Added example well completion diagrams ass additional non-mandatory information in an Appendix. Examples from two states and one federal agency where shown.